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REVISIONS

NO.	DATE	DESCRIPTION

GENERAL NOTES

1. Design is based on
   a) State of CONNECTICUT Building Code
   b) 6000 psf soil pressure
   c) 2500 psi concrete at 28 days for footings
   d) 3000 psi concrete at 28 days for all other work
   e) SEISMIC ZONE C (ZONE 2 - 1990)

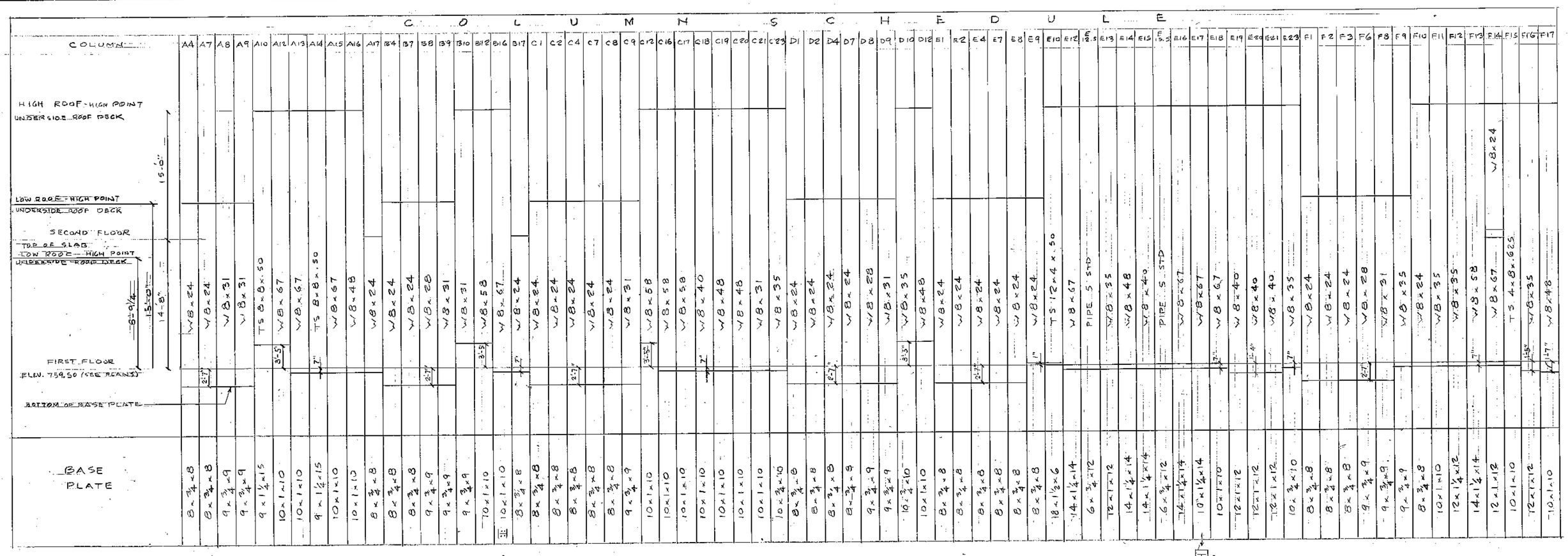
   WIND: 85 MPH
   ROOF LIVE LOAD: 85 PSF
   FLOOR LIVE LOADS: CLASSROOM - 40 PSF - CORRIDOR - 80 PSF
2. Place approved granular backfill under slabs and footings as necessary in 6 inch layers and compact and test each layer to 95% density as defined by the ASTM Test Designation 1557-Method D.
3. Reinforcing steel to be deformed bars of new billet steel as per ASTM Specification A615-60.
4. All concrete work to conform to latest ACI Code 318.
5. Wire fabric to be cold drawn electrically and to conform to ASTM Specification A-185.
6. Wire fabric to be lapped not less than one mesh.
7. All continuous reinforcing to be lapped 36 bar diameters unless otherwise shown or indicated.
8. Protection of reinforcing to be as follows unless otherwise shown

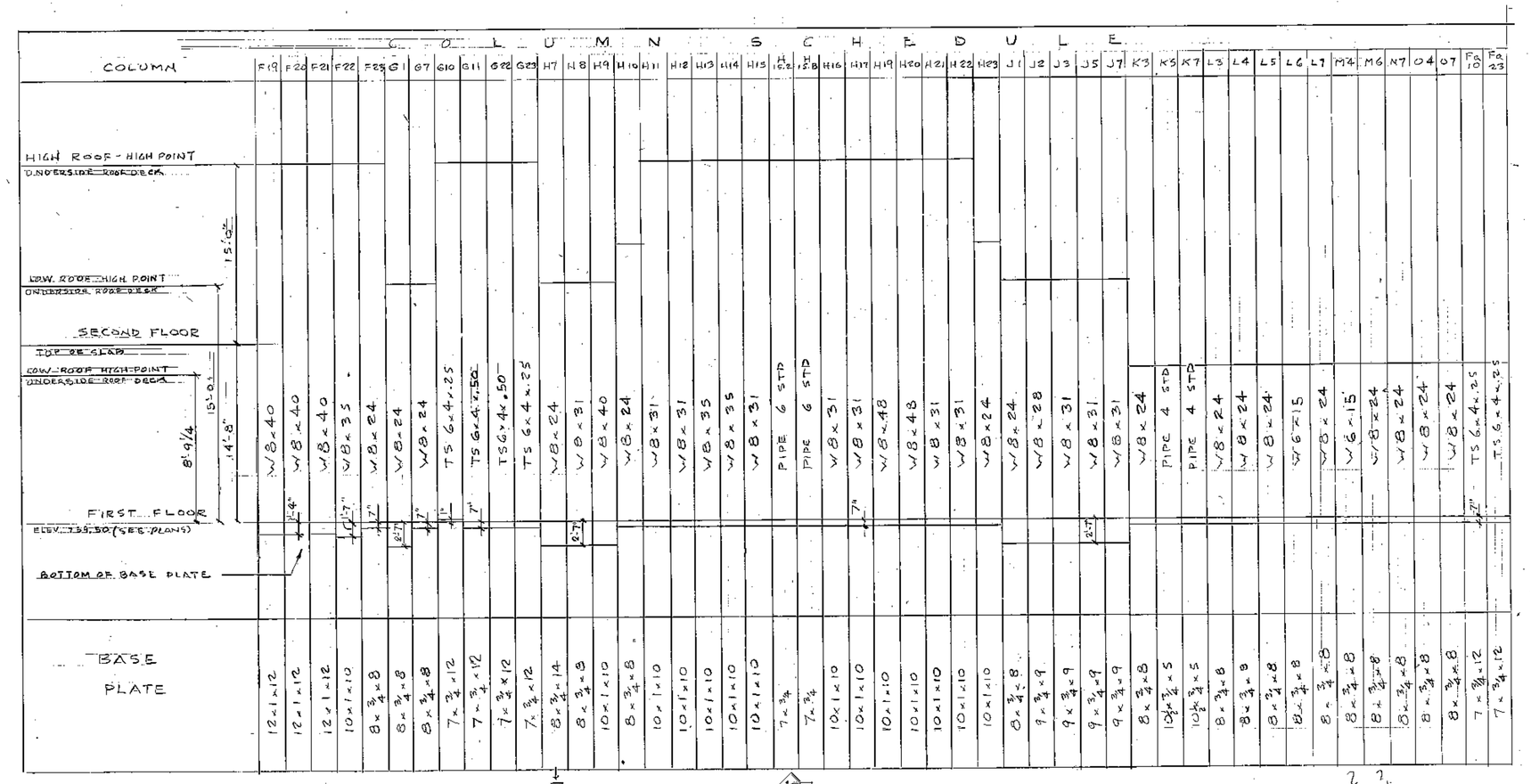
Footings and slabs on earth	3" clear
Piers and walls exposed to weather	2" clear
Slabs not exposed to weather	3/4" clear

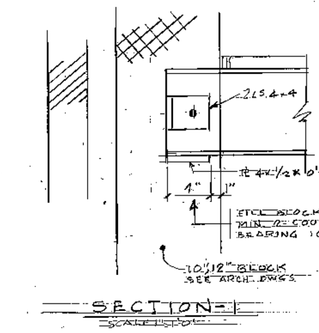
9. Elevation of bottom of footings shown thus ( ). Bottom of all footing elevations subject to revision as exposed conditions warrant.
10. All slabs to be poured with keyed edges in sections of 60 ft. maximum. Contraction joints to be at 20 ft. on center maximum. At construction joints thicken slab to 6 inches and provide dowels, #3 x 4'-0" at 18" on center.
11. See Architectural drawings for brick or block ledges, limits of depressed slabs and additional details.
12. See Electrical, Plumbing, H&V drawings location and size of sleeves, slots, and inserts in walls and slabs, concrete pads, etc.
13. All framing and and walls shall be adequately braced during construction, against movement in any direction, at any level, until lateral support is provided by other required construction.
14. Fabrication and erection of all structural steel work shall conform to the Specifications of the American Institute of Steel Construction for structural steel; and AISC "Code of Standard Practice for Steel Buildings and Bridges"

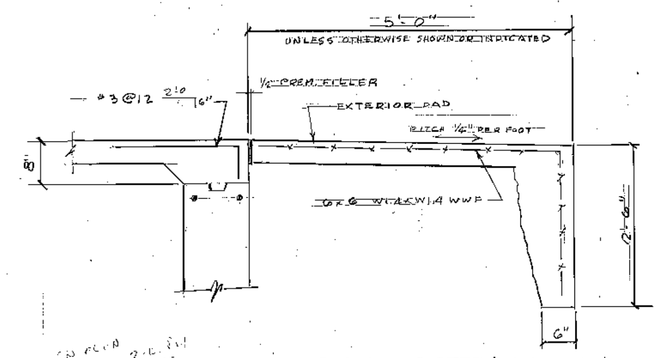
    Paragraph 4.2.1 of the above code is hereby modified by deletion of the following sentence: "This approval constitutes the owner's acceptance of all responsibility for the design adequacy of any connections designed by the fabricator as a part of his preparation of these shop drawings".

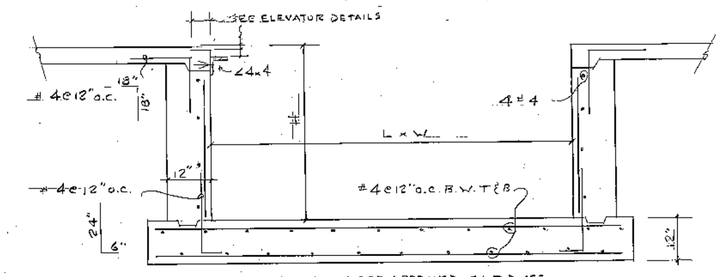
    AISC "Specifications for the Design, Fabrication, and Erection of Structural Steel for Buildings", including the "Commentary and Supplements thereto as issued.
15. All steel shall be new steel and conform to ASTM Designation A-36
16. Beam connections
    a) All beam connections, both shop and field, shall be framed beam connections and heavy framed beam connections, as per Part 4 of the Manual of Steel Construction of the AISC.
    b) All beams that framed into column webs shall have seated connections with top angle or side angle.
    c) The connection for all beams shall be adequate to carry their reaction due to a maximum uniformly distributed load that the beam is capable of carrying for it's span based on the allowable unit stresses of the AISC Building Code as revised to date. Connections shall be adequate for increased reactions due to concentrated loads near end of beams.
17. All welding shall conform to the Code of Arc and Gas Welding in Building Construction of the American Welding Society.
18. Unauthorized splicing of structural steel will not be permitted.
19. Bolt all joists at or near columns. Extend and weld full bottom chords at these points.
20. Provide galvanized sloping recessed sump pans at all roof drains.
21. Suspended or supported material of any kind shall not be carried by the roof deck.
22. Contractor to verify all dimensions and conditions in the field. Engineer to be notified immediately of any variations or discrepancies.



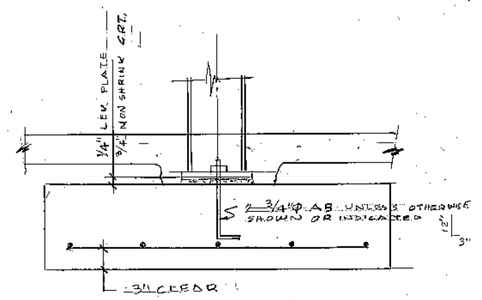




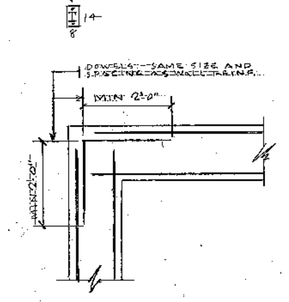


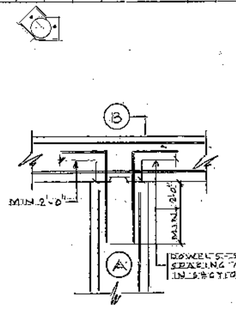


ELEVATOR PIT SECTION
SCALE: ½" = 1'-0"

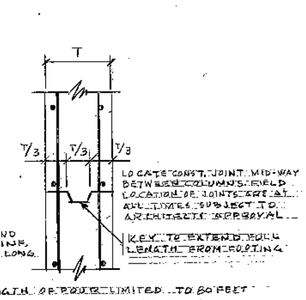


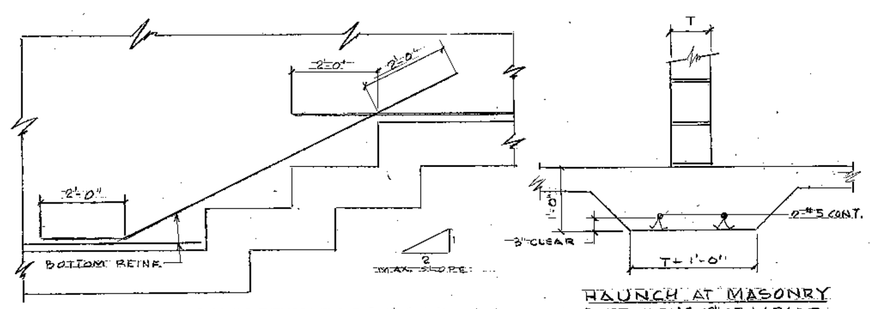
TYPICAL COLUMN BASE AND FOOTING DETAIL





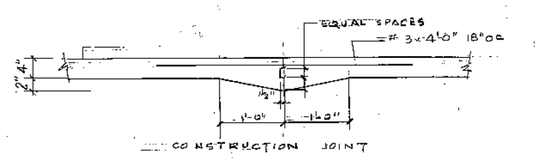
TYPICAL CONCRETE WALL DETAILS

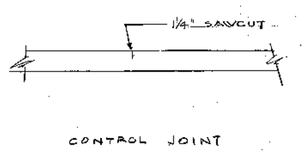


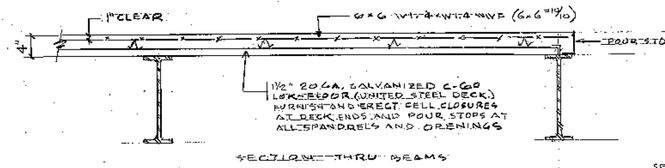


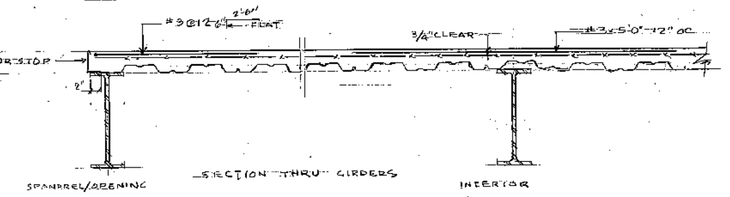
TYPICAL STEPPED FOOTING DETAIL

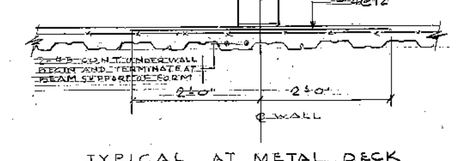
RAUNCH AT MASONRY PARTITIONS 8" OR LARGER











TYPICAL AT METAL DECK PARALLEL TO MASONRY PARTITION

TYPICAL CONCRETE SLAB ON METAL FORM DETAILS

LINTEL SCHEDULE (UNLESS SHOWN OTHERWISE)	
MASRY OPNG	SIZE PER 4" MASONRY
UP TO 4'-6"	L 3½ x 3½ x 5/16
4'-7" TO 6'-6"	L 4 x 3½ x 5/16
6'-7" TO 9'-0"	L 5 x 3½ x 3/8
6" PART TO 9'-0"	WT 7 x 13 OR L 6 x 6 x 3/8

SEE ARCH. DWGS FOR FINISH PLATES REQ'D. THICKNESS (WALL THICKNESS -1") x 3/16" (M.O. ONLY)

I EXISTING W8 I IR CAPTURE E-70 - ADD NEW R - 13-1/4-8-7 R WELD NOW TO 13½ S REQUIRED VERIFY LENGTH

THE LAWRENCE ASSOCIATES
1095 TOLLAND TURNPIKE MANCHESTER, CONNECTICUT 06040 (203) 643-2591

PROJECT NUMBER 2155.0 012-89-027Pa

PROJECT TITLE
ALTERATIONS AND ADDITIONS TO :
BOLTON ELEMENTARY SCHOOL
NOTCH ROAD
BOLTON, CONNECTICUT

DRAWING TITLE
COLUMN SCHEDULE, DETAILS, NOTES.

SCALE

DATE 8-17-1992

S-6

DRAWING 5D OF 89